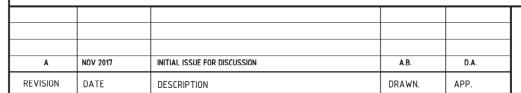


JOINT TO BE SAWN AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY THAT IT WILL NOT BE DAMAGED BY SAWING (MAX 24HRS)

REINFORCED FOOTPATH CONTRACTION JOINT (CJ) OPTIONS





| FOOTPATH DETA | AILS - REINFO | RCED | SCALE NTS |
|---------------|---------------|-------------|--------------|
| APPROVED | DATE | DRAWING No. | REV |
| D.A. | NOV 2017 | SD04 | Α |

SECTION A - A

NOTES:

2% CROSSFALL

CONCRETE SHALL BE MINIMUM 32MPa COMPRESSIVE STRENGTH (f'c) AT 28 DAYS, 40mm COVER.

SL72 MESH, 40mm COVER

- REINFORCED FOOTPATH TO BE 1200mm WIDE, 100mm THICK PLAIN CONCRETE ON 50mm GRANULAR BEDDING LAYER
- GRANULAR BEDDING LAYER TO BE PLACED ON SUITABLE, COMPACTED SUBGRADE. UNSUITABLE SUBGRADE IS TO BE REMOVED AND REPLACED AS REQUIRED BY COUNCIL.

32MPa, 100mm THICK CONCRETE FOOTPATH WITH NON-SLIP FINISH

MIN. 12mm WIDE COMPRESSIBLE FILLER

MATERIAL, FULL DEPTH. (EG.

IMPREGNATED CORK, MASTIC, ABLEFLEX ETC)

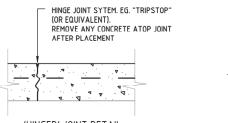
ISOLATION JOINT DETAIL (IJ)

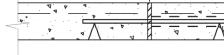
DOWELLED EXPANSION JOINT

DOWELL MAX SPACING 300mm

"CONNOLLY XJ100" OR EQUIVALENT

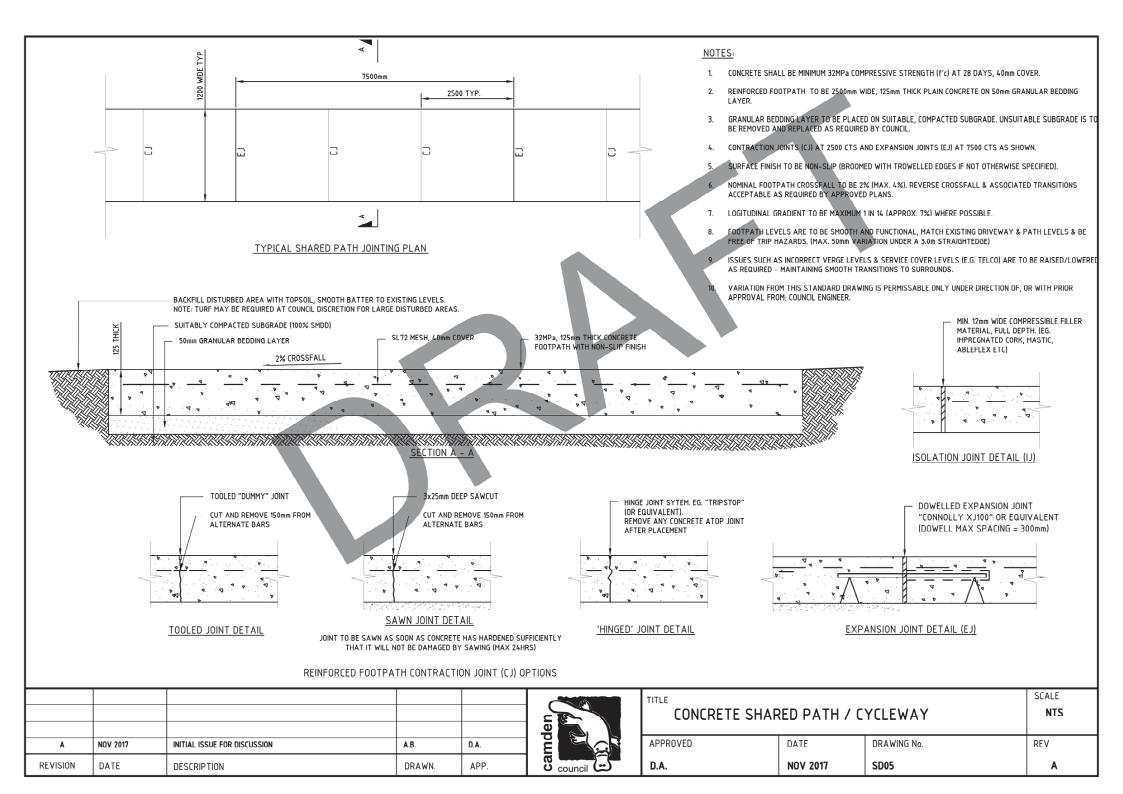
- CONTRACTION JOINTS (CJ) AT 1200 CTS AND EXPANSION JOINTS (EJ) AT 6000 CTS AS SHOWN.
- SURFACE FINISH TO BE NON-SLIP (BROOMED WITH TROWELLED EDGES IF NOT OTHERWISE SPECIFIED).
- NOMINAL FOOTPATH CROSSFALL TO BE 2% (MAX. 4%). REVERSE CROSSFALL & ASSOCIATED TRANSITIONS ACCEPTABLE AS REQUIRED BY APPROVED PLANS.
- LOGITUDINAL GRADIENT TO BE MAXIMUM 1 IN 14 (APPROX. 7%) WHERE POSSIBLE.
- FOOTPATH LEVELS ARE TO BE SMOOTH AND FUNCTIONAL, MATCH EXISTING DRIVEWAY & PATH LEVELS & BE FREE OF TRIP HAZARDS. (MAX. 50mm VARIATION UNDER A 3.0m STRAIGHTEDGE)
- ISSUES SUCH AS INCORRECT VERGE LEVELS & SERVICE COVER LEVELS (E.G. TELCO) ARE TO BE RAISED/LOWERED AS REQUIRED MAINTAINING SMOOTH TRANSITIONS TO SURROUNDS.
- 10. VARIATION FROM THIS STANDARD DRAWING IS PERMISSABLE ONLY UNDER DIRECTION OF, OR WITH PRIOR APPROVAL FROM COUNCIL ENGINEER.

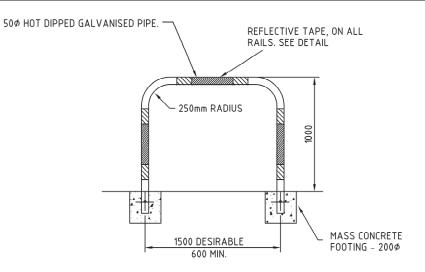




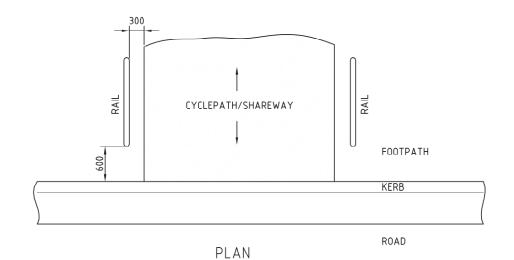
EXPANSION JOINT DETAIL (EJ)

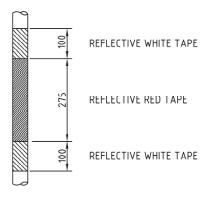
'HINGED' JOINT DETAIL





ELEVATION





REFLECTIVE TAPE DETAIL

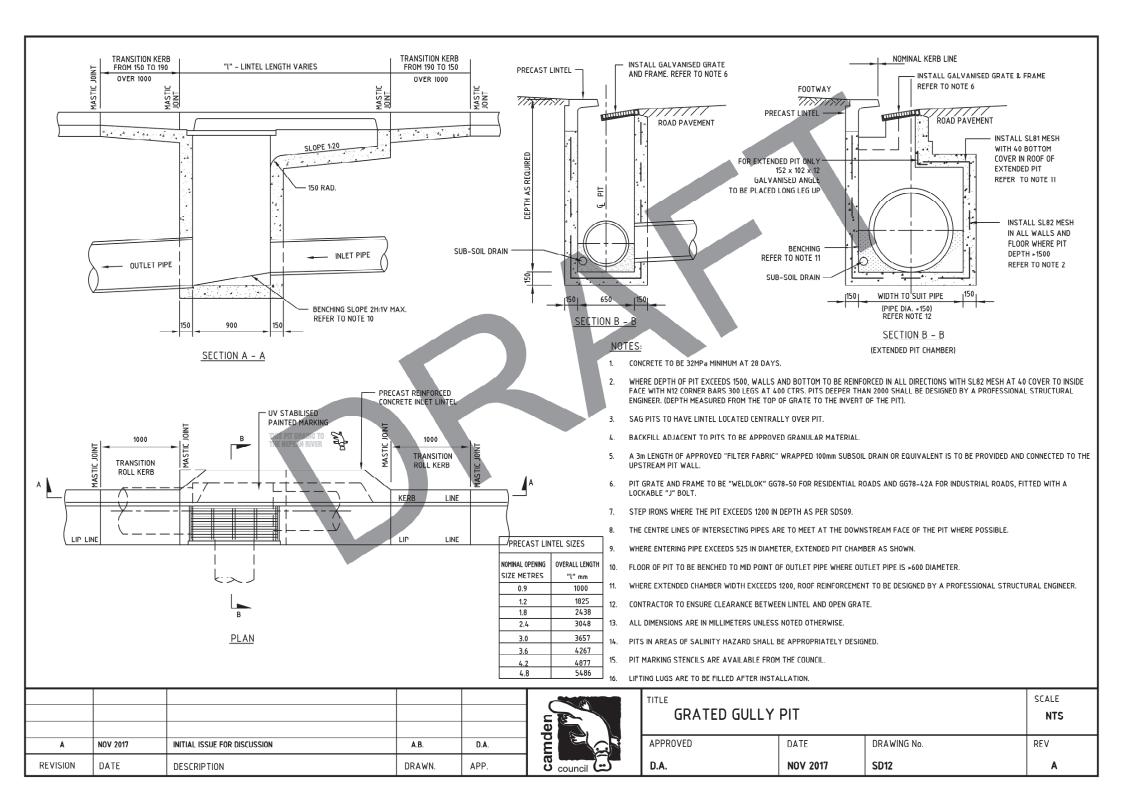
NOTES:

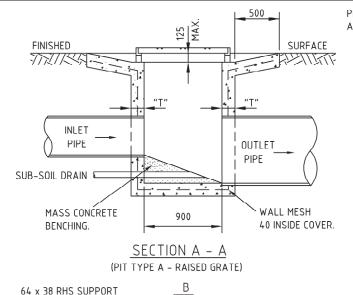
- 1. ALL DIMENSIONS ARE IN MILLIMETRES
- 2. PLACE N12 BAR 150mm LONG IN HOLE BEFORE FIXING INTO PLACE
- 3. CONCRETE TO BE 25MPa @ 28 DAYS.
- 4. ALL STEELWORK TO BE HOT DIPPED GALVANISED.
- WHERE DIRECTED BY THE PCA/COUNCIL, ADDITIONAL/ALTERNATE RAILS MAY BE REQUIRED
- ANY RAIL INSTALLATION SHALL BE IN ACCORDANCE WITH AUSTROADS PART 14 - BICYCLES.
- REFLECTIVE TAPE TO BE CLASS 1 (AS1906.1).

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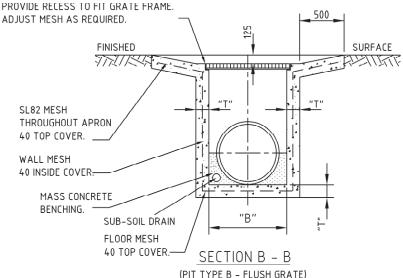
| TITLE | SCALE | | |
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| PATHWAY RA | NTS | | |
| APPROVED | DATE | DRAWING No. | REV |
| C. McINTYRE | JAN 2009 | SD06 | А |





REFER DETAIL. (TYPE A)

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| PIT DIMENSIONS and MESH | | | | |
|----------------------------------|--------------------------------|-----|-------|--|
| LARGEST PIPE CONNECTED TO PIT | MESH (WALLS and FLOOR ONLY) | | | |
| UP TO 525 | 600 | 150 | SL82 | |
| UP TO 750 | 900 | 150 | SL82 | |
| 825 - 900 | 1000 | 150 | SL82 | |
| 1050 - 1200 | 1400 | 150 | SL82 | |
| 1350 | 1550 | 200 | SL82 | |
| 1500 | 1700 | 200 | SL102 | |

NOTWITHSTANDING THE ABOVE TABLE, PITS DEEPER THAN 2000 TO INVERT SHALL BE STRUCTURALLY DESIGNED.

- WELDLOK OR EQUIVALENT
HOT DIPPED GALVANISED STEEL
GRATE WITH HINGED ENTRY AND LOCK
DOWN "J" BOLT TO MANUFACTURER'S
DETAIL.

PLAN - PIT TYPE A and B

В

900

(SUIT PIPES UP TO 1500mm DIA.)

PIT TYPE C - V GRATE FOR USE IN DISH CROSSINGS AND TABLE DRAINS

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| TITLE | | | SCALE |
|-------------------|----------|-------------|-------|
| SURFACE INLET PIT | | | |
| APPROVED | DATE | DRAWING No. | REV |
| C. McINTYRE | JAN 2009 | SD13 | А |

NOTES

FINISHED

CONCRETE TO BE 32MPa MINIMUM AT 28 DAYS.

SECTION B - B

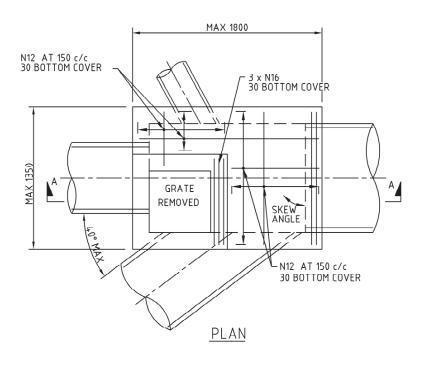
(PIT TYPE C - V GRATE)

GRATE NOT TO EXTEND

BEYOND LIP LINE OF KERB

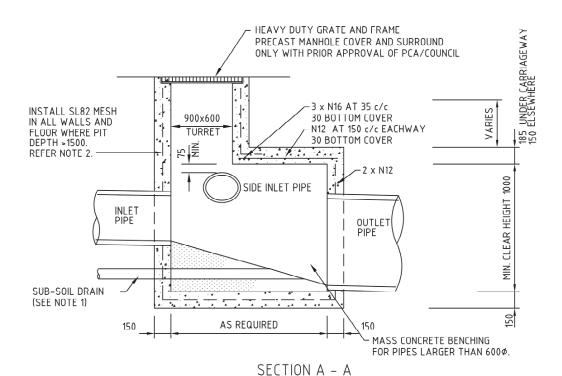
SURFACE

- APPROVED STEP IRONS SHALL BE
 PROVIDED WHERE THE PIT EXCEEDS 1200
 IN DEPTH. REFER TO SD20
- GRATES MUST BE CLASS C FOR NON-ROAD INSTALLATION AND CLASS D FOR ROAD INSTALLATION. CLASSES AS DEFINED IN A.S.3996.
- GRATE LEGS TO BE WELDED TO FRAME PRIOR TO GALVANISING. (TYPE A)
- ALL CONCRETE WORK TO BE A MINIMUM OF 150 THICK.
- MASS CONCRETE BENCHING TO PIPE CENTRELINE MUST BE PROVIDED AS INDICATED.
- 7. WHERE SITE CONDITIONS DICTATE, THE THE PCA/COUNCIL MAY INCLINE THE PIT TOPS TO AN UPPER LIMIT OF 1 VERT. in 4 HORIZ. NO ALTERATION TO REINFORCEMENT IS REQUIRED, HOWEVER, THE ENTIRE PIT ROOF (AND ACCOMPANYING APRONS) ARE TO REMAIN PLANAR.
- ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE NOTED.
- PITS IN AREAS OF SALINITY HAZARD SHALL BE APPROPRIATELY DESIGNED.



NOTES:

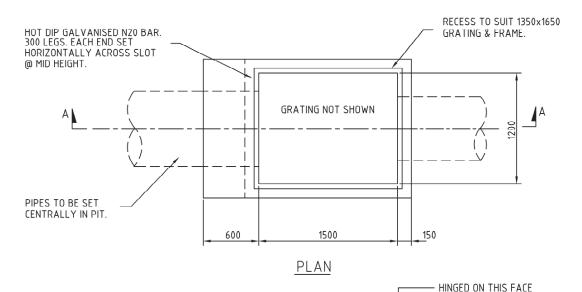
- PROVIDE Ø100 SUB-SOIL DRAINAGE PIPE, 3000 LONG, WRAPPED IN FABRIC SOCK ADJACENT TO INLET PIPES.
- 2. MAXIMUM OUTLET PIPE ON STRAIGHT Ø900
- 3. MAXIMUM OUTLET PIPE ON SKEW Ø825
- 4. MAXIMUM SIDE ENTRY PIPE Φ825 AT APPROX. 40° SKEW.
- 5. MINIMUM INTERNAL DIMENSIONS LENGTH 900
 - WIDTH 700
 - HEIGHT 1000.
- CAST IN SITU CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 32MPa AT 28 DAYS.
- WHERE PITS ARE DEEPER THAN 1200 PROVIDE STEP IRONS. REFER TO SD20
- PITS DEEPER THAN 1500 SHALL BE REINFORCED WITH ONE LAYER OF SL82 TO FLOOR AND WALLS FOR THE FULL DEPTH. PITS DEEPER THAN 2000 SHALL BE STRUCTURALLY DESIGNED.
- PITS IN AREAS OF SALINITY HAZARD SHALL BE APPROPRIATELY DESIGNED.

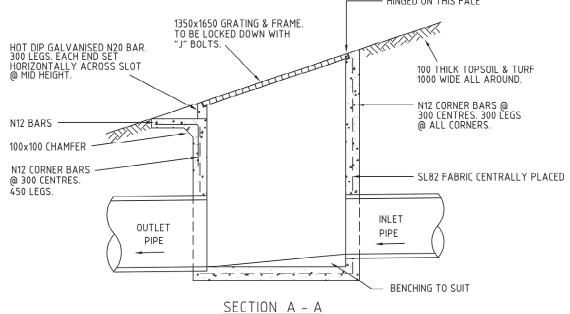


A JAN 2009 FIRST ISSUE A.P. C.M.
REVISION DATE DESCRIPTION DRAWN. APP.



| I | TITLE | | | |
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| | JUNCTION PIT | | | |
| | APPROVED | DATE | DRAWING No. | REV |
| | C. McINTYRE | JAN 2009 | SD14 | А |





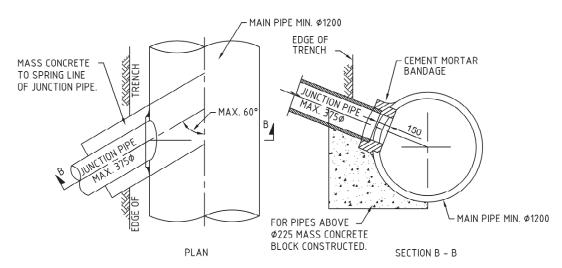
NOTES

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. CONCRETE TO BE 32MPa @ 28 DAYS.
- 3. ALL REINFORCEMENT TO BE CENTRALLY LOCATED.
- 4. ALL PIT WALLS AND BASE TO BE MIN 150 THICK.
- 5. STREAMLINE BASE OF PIT TO SUIT.
- PITS GREATER THAN 2000 IN DEPTH SHALL BE DESIGNED BY A PROFESSIONAL STRUCTURAL ENGINEER.
- WHERE DIRECTED PROVIDE 3000 OF 100¢ CORRUGATED P.V.C. SUBSOIL DRAIN ON INLET SIDE OF PIT. SUBSOIL DRAIN TO BE SURROUNDED BY MIN 300 OF 5mm AGGREGATE.
- 8. PROVIDE STEP IRONS AS PER SD20 FOR PITS DEEPER THAN 1200.
- 9. PITS IN AREAS OF SALINITY HAZARDS SHALL BE APPROPRIATELY DESIGNED.

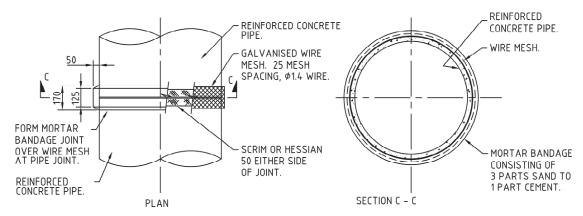
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| REVISION | DATE | DESCRIPTION | DRAWN. | APP. |



| TITLE | SCALE | | |
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| SURCHARGE P | NTS | | |
| APPROVED | DATE | DRAWING No. | REV |
| C. McINTYRE | JAN 2009 | SD16 | A |



TYPICAL DIRECT CONNECTION OF SMALL PIPE AND LARGE PIPE



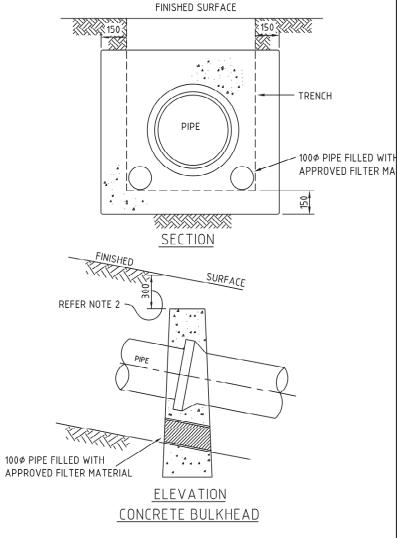
BANDAGE JOINT DETAILS

NOTES:

- COMPRESSIVE STRENGTH F'c FOR CAST IN SITU CONCRETE TO BE A MIN. 32 MPa AT 28 DAYS.
- 2. ALL DIMENSION ARE IN MILLIMETRES UNLESS OTHERWISE NOTED.

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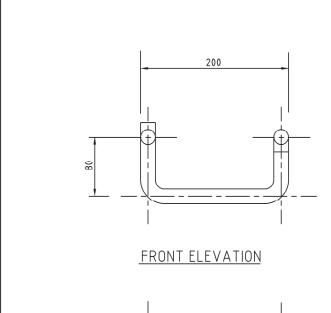


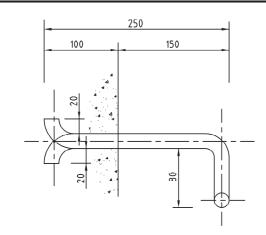


NOTES:

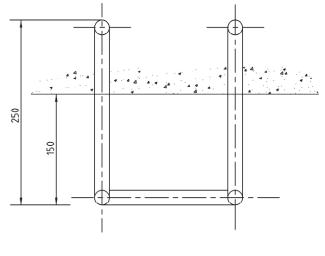
- COMPRESSIVE STRENGTH F'c FOR CAST IN SITU CONCRETE TO BE A MIN. 32 MPa AT 28 DAYS.
- 2. WHERE THE PIPELINE IS UNDER ROAD PAVEMENT THE TOP OF THE BULKHEAD SHALL EXTEND TO THE SUBGRADE LEVEL.
- BULKHEADS ARE TO BE PLACED AT EACH COLLAR JOINT FOR PIPELINE GRADES GREATER THAN 16%

| TITLE | SCALE | | |
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| C. McINTYRE | JAN 2009 | SD17 | А |





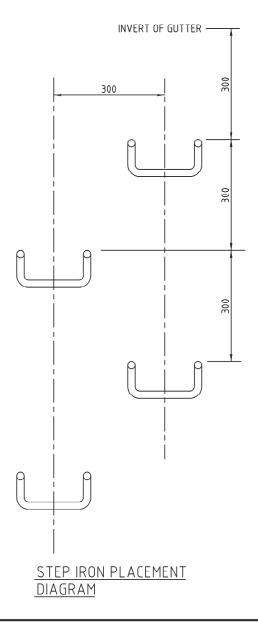
SIDE ELEVATION



PLAN

NOTES

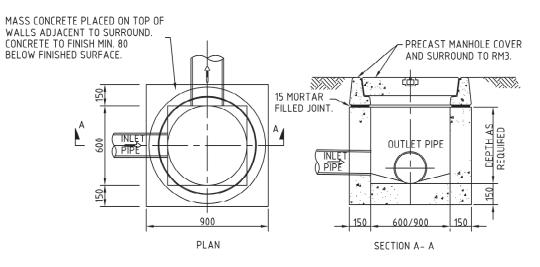
- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. STEP IRONS TO BE FABRICATED FROM \$\phi 20 M.S. BARS
- 3. ALL BENDS TO BE FORMED AROUND \$\phi\$12 PIN
- 4. STEP IRONS TO BE HOT DIPPED GALVANISED
- PITS LESS THAN 1200mm (MEASURED FROM THE TOP OF KERB) TO HAVE ONE STEP IRON LOCATED 600mm BELOW INVERT OF GUTTER



| А | JAN 2009 | FIRST ISSUE | A.P. | C.M. |
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| TITLE | | | SCALE |
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| STEP IRONS | | | NTS |
| APPROVED | DATE | DRAWING No. | REV |
| C. McINTYRE | JAN 2009 | SD20 | А |



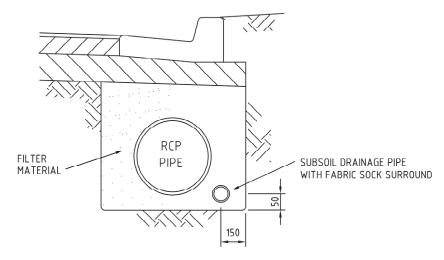
CONCRETE INSPECTION PIT

THE INTERNAL JUNCTION SHALL BE SMOOTHLY FINISHED WITH 2:1 CEMENT MORTAR SO AS TO PRESENT NO OBSTRUCTION WITHIN THE INTERNAL CHANNEL OF THE R.C. STORMWATER PIPE.

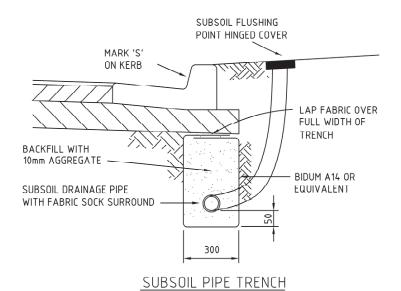
| А | NOV 2006 | FIRST ISSUE DRAFT | A.P. | C.M. |
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| REVISION | DATE | DESCRIPTION | DRAWN. | APP. |

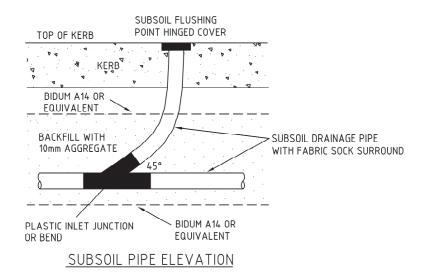


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| CONCRETE INS | NTS | | |
| APPROVED | DATE | DRAWING No. | REV |
| C. McINTYRE | NOV 2006 | SD22 | A |



SUBSOIL PIPE IN STORMWATER DRAINAGE TRENCH





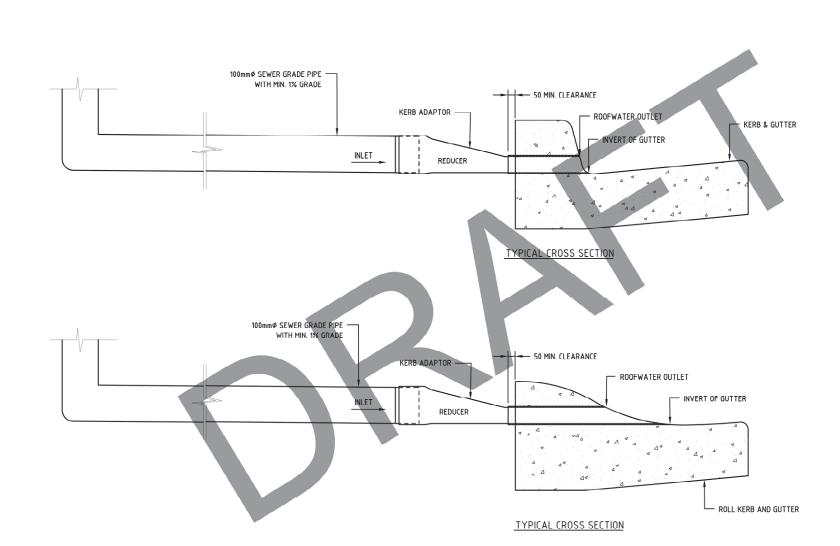
NOTES.

- 1. ALL SUBSOIL PIPE TO BE 100\(\phi\) MIN. SLOTTED PLASTIC
- 2. MINIMUM GRADE OF SUBSOIL TO BE 1%
- 3. SUBSOIL FLUSHING POINTS TO BE NOT MORE THAN 60 METRES INTERVALS
- 4. JOINTS IN FILTER FABRIC TO BE LAPPED MINIMUM 300mm
- WHERE LOCATED UNDER ROAD PAVEMENT, SUBSOIL TRENCH IS TO BE AT SUBGRADE LEVEL
- SUBSOIL DRAINS TO BE CONTINUED HARD UP AGAINST WALLS OF DRAINAGE PITS WITH SLOTTED PLASTIC DRAIN EXTENDING THROUGH THE UOSTREAM
- 7. SUBSOIL DRAINS ARE TO BE EVENLY GRADED WITH NO DEPRESSIONS
- 8. IN AREAS OF PERVIOUS SUBGRADE OR FRAGMENTED MATERIAL, THE BOTTOM OF THE TRENCH IS TO BE LINED WITH A SUITABLE PLASTIC MEMBRANE RETURNED 200mm UP EACH SIDE OF THE TRENCH

| А | NOV 2006 | FIRST ISSUE DRAFT | A.P. | C.M. | | |
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| TITLE | SCALE | | |
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| SUBSOIL DRAI | NTS | | |
| APPROVED DATE DRAWING No. | | | REV |
| C. McINTYRE | NOV 2006 | SD100 | А |



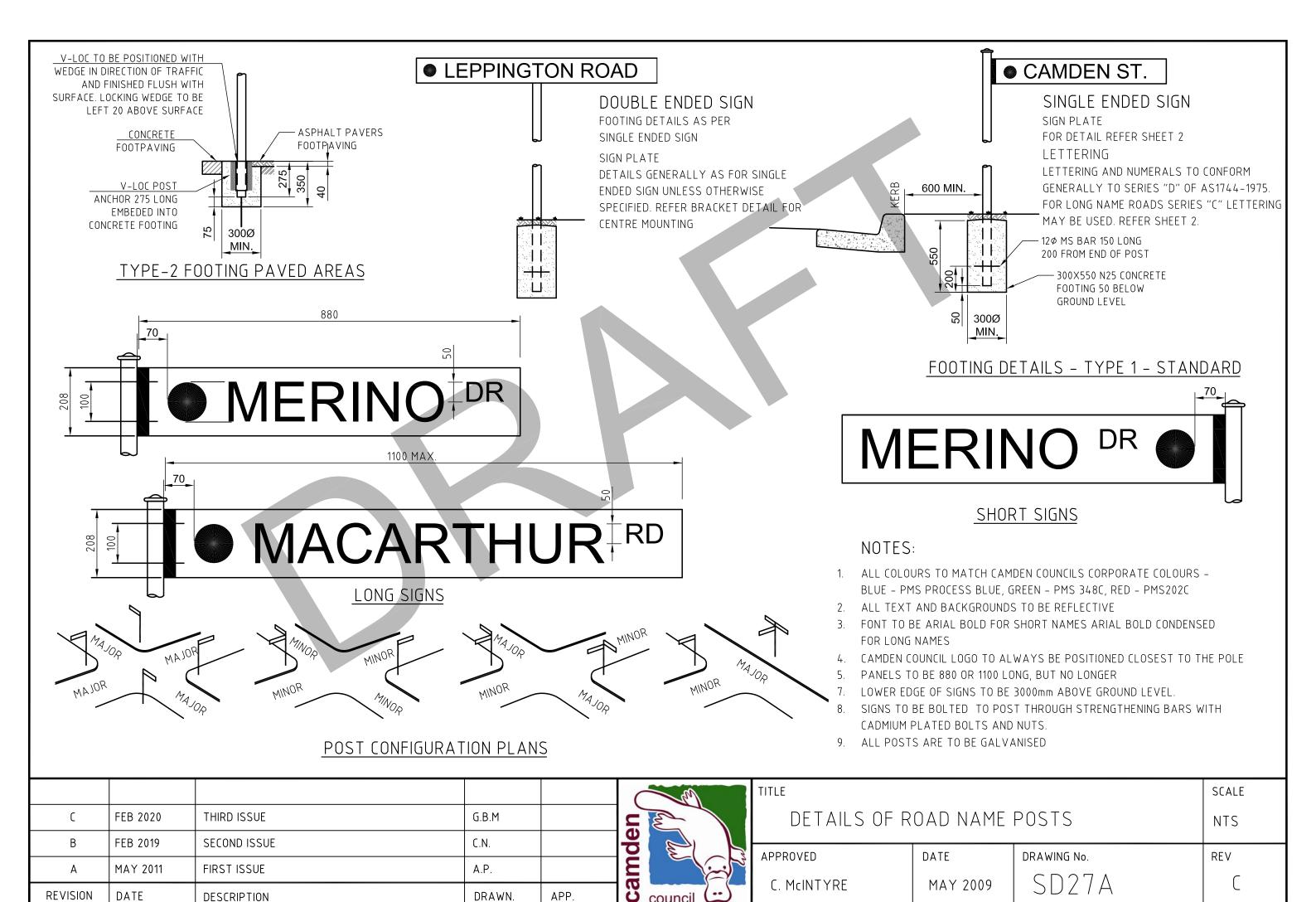
NOTES:

- 1. KERB OUTLETS TO BE MANUFACTURED FROM 150X50X4 mm HOT DIPPED GAL.MILD STEEL TO SUIT KERB TYPE, AND TO EXTEND MIN, 50mm BEHIND REAR OF KERB.
- 2. SEAL JOINT BETWEEN ADAPTOR AND OUTLET WITH APPROVED SILICON SEALANT.
- 3. FOR ALL OUTLETS, ADAPTORS AND REDUCERS TO BE FROM AN APPROVED MANUFACTURER AND ALL JOINTS TO BE SEALED & WATERTIGHT.

| A | NOV 2017 | INITIAL ISSUE FOR DISCUSSION | A.B. | D.A. |
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| REVISION | DATE | DESCRIPTION | DRAWN. | APP. |



| KERB ADAPTOR | SCALE NTS | | |
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| D.A. | NOV 2017 | SD26 | Α |



council

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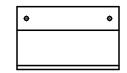
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REVISION

DATE

DESCRIPTION





DETAIL 1

BRACKET FOR ATTACHING SINGLE SIGN TO STEEL POST



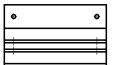
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DETAIL 5

BRACKET FOR ATTACHING

DOUBLE ENDED SIGN

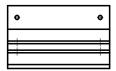
POSTS OR CENTRE MOUNTING

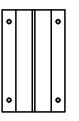


BRACKET FOR ATTACHING

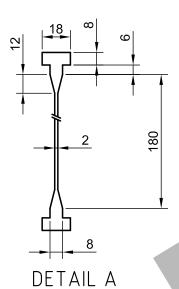
TWO SIGNS TO STEEL POST

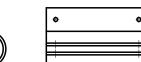




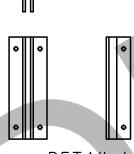


DETAIL 3 BRACKET FOR ATTACHING THREE SIGNS TO STEEL POST





DETAIL 2



BRACKET FOR ATTACHING

1.0 SIGN BLADE

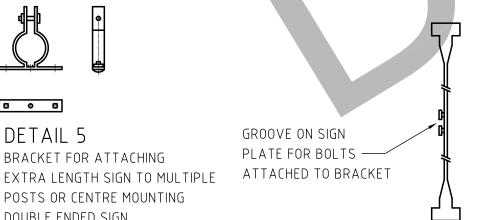
2.1 SIGNS SHALL BE MANUFACTURED FROM AN APPROVED MARINE GRADE HIGH TENSILE STRENGTH ALUMINIUM EXTRUSION WITH SQUARE END.

2.2 THE LENGTH OF SIGNS SHALL NOT BE MORE THAN 1100mm 2.3 THE FLANGES SHALL BE 18mm MINIMUM WIDTH AND NO LESS THAN 6mm THICK. THE FILLET SHALL BE NOT LESS THAN 5mm WIDE AND 7mm MINIMUM THICKNESS AND THE WEB SHALL BE 2mm MINIMUM THICKNESS (REFER DETAIL A).

2.4 THE BLADES SHALL BE PRE-DRILLED TO COINCIDE WITH THE POSITION AND DIAMETER OF THE BRACKETS. THE BLADES SHALL BE 202mm BLADES.

DETAIL 4

TIMBER SIGN TO TIMBER POST



2.0 STRFFT NAME SIGNS

3.1 BACKGROUND

THE BACKGROUND SHEETING SHALL BE "CLASS 1 REFLECTIVE WHITE" VINYL SHEETING BONDED TO THE PREPARED ALUMINIUM EXTRUSION BLADE BY THE APPROVED METHOD TO MEET AS/NZ 1906.1.2017 APPLICABLE FOR A 8 YEAR DURABILITY WARRENTY. THE SHEETING SHALL EXTEND FOR THE FULL LENGTH OF THE BLADE AND BE UNSPLICED ALONG ITS ENTIRE LENGTH. THE MINIMUM WIDTH OF THE BACKGROUND MATERIAL SHALL BE 180mm FOR 202mm BLADES.

3.0 BRACKETS

4.1 BRACKETS SHALL BE COMPATIBLE WITH THE SPECIFIED BLADE. 4.2 BRACKETS SHALL BE PRE-DRILLED (10mm DIA). BRACKETS SHALL BE SUPPLIED COMPATIBLE WITH BOLTS, NUTS AND WASHERS. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND OF SUITABLE SIZE.

4.0 TOLERANCE

MANUFACTURING TOLERANCES OF SIGNS SHALL BE SPECIFIED IN SECTION 4 MANUFACTURING AND MATERIALS OF A.S. 1743.2018 SPECIFICATIONS.

5.0 PROTECTION AND PACKAGING

PROTECTION AND PACKAGING OF ALL BLADES, AND ASSOCIATED FITTINGS FOR DELIVERY TO COUNCIL SHALL BE THE RESPONSIBILITY OF THE SUPPLIER. SIGNS ARE TO BE PROTECTED FROM DAMAGE DURING TRANSIT BY APPROVED WRAPPING PRIOR TO DELIVERY.

6.0 WARRANTY

ALL STREET SIGN BLADES SHALL CARRY A 8 YEAR PERFORMANCE WARRENTY FROM THE RETRO-REFLECTIVE MATERIAL. MANUFACTURER DETAILS OF THE PERFORMANCE WARRENTY SHALL BE SUBMITTED WITH THE QUOTATION/TENDER.

NOTE

- 1. CONTRACT SIGN MANUFACTURERS TO OBTAIN COUNCIL LOGO AND SPECIFICATIONS FROM COUNCIL
- 2. END MOUNTED SIGNS MUST HAVE STREET NAME ON BOTH SIDES.
- 3. DOUBLE ENDED SIGNS MUST HAVE STREET NAMES ON ONE SIDE ONLY.

| В | FEB 2020 | SECOND ISSUE DRAFT | G.B.M | |
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| А | FEB 2019 | FIRST ISSUE DRAFT | C.N. | C.M. |
| REVISION | DATE | DESCRIPTION | DRAWN. | APP. |



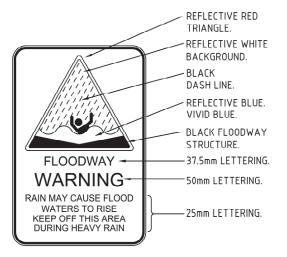
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| DETAILS OF B | NTS | | |
| APPROVED | REV | | |
| C. McINTYRE | NOV 2006 | SD27B | В |







- SIGN TO CONSIST OF 3mm THICK ALUMINIUM & TO BE IN CORPORATE COLOURS ON A DIAMOND GRADE, WHITE REFLECTIVE PLATE, UV TREATED COVERED IN ANTI-GRAFFITI FILM AND COMPLY WITH A.S.1743.
- 2. LETTERING "WARNING" TO BE RED AND OTHERS IN BLACK.
- THE GRAGHIC CONSISTS OF AN OUTLINE SKETCH OF THE PEDESTRIAN AND A RED SYMBOLIC SIGN "PROHIBITED ENTRY" OVER THE SKETCH.
- 4. COLOUR OF GRAPHIC SHALL BE BLACK.
- 5. THE SIGN SHALL BE 900 HIGH BY 600 WIDE WITH CORNER RADII OF 60.
- 6. SIGN POST SHALL BE HOT DIPPED GALVANISED, 101.6 x 5.0 CHS, 3100 LONG WITH HOT DIPPED GALVANISED TOP END CAP. POST TO BE SET 600 DEEP INTO Ø300 MASS CONCRETE. GROUND CLEARANCE TO SIGN TO BE MINIMUM 2000.



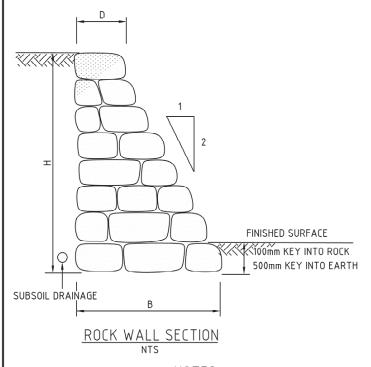
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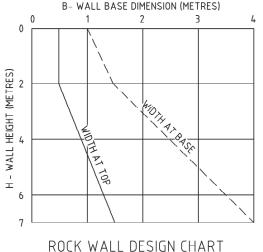
- SIGN TO CONSIST OF 3mm THICK ALUMINIUM & TO BE IN CORPORATE COLOURS ON A DIAMOND GRADE, WHITE REFLECTIVE PLATE, UV TREATED COVERED IN ANTI-GRAFFITI FILM AND COMPLY WITH A.S.1743.
- 2. LETTERING "WARNING" TO BE RED AND OTHERS IN BLACK.
- 3. THE SIGN SHALL BE 900 HIGH BY 600 WIDE WITH CORNER RADII OF 60.
- 4. SIGN POST SHALL BE HOT DIPPED GALVANISED, 101.6 x 5.0 CHS, 3100 LONG WITH HOT DIPPED GALVANISED TOP END CAP. POST TO BE SET 600 DEEP INTO Ø300 MASS CONCRETE. GROUND CLEARANCE TO SIGN TO BE MINIMUM 2000.

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NOTES

- 1. THE NORMAL LOADING CONDITION ASSUMES THAT THE WATER TABLE IS BELOW THE BASE OF THE WALL
- 2. BACKFILL MATERIALS TO BE ESSENTIALLY GRANULAR FREE DRAINING MATERIAL;
- 3. THE RETAINED SURFACE IS HORIZONTAL
- 4. WHERE THE SURFACE SLOPE OF THE RETAINED MATERIAL IS BETWEEN 1(V) TO 10(H), THE WALL BASE DIMENSION IS TO BE INCREASED BY 0.5M.
- 5. WHERE THE NATURAL SURFACE BEHIND THE WALL IS TO CARRY SURCHARGE LOADING THE WALL GEOMETRY SHALL BE AMENDED TO ALLOW FOR AN EQUIVALENT WALL HEIGHT EQUAL TO THE TRUE HEIGHT PLUS 0.5M FOR EACH 10KPA OF SURCHARGE. A 20KPA SURCHARGE ADEQUATELY COVERS THE B99 VEHICLE (AS/NZS 2830), FULLY LADEN. ALL SURCHARGES IN EXCESS OF 20KP WILL REQUIRE SITE SPECIFIC DESIGN.
- 6. WHERE WALL HEIGHTS EXCEED 2.5M THE WALL SHALL BE FOUNDED ON BEDROCK. WALLS OF LESS THAN 2.5M IN HEIGHT MAY BE PLACED ON SUITABLE SOIL FOUNDATIONS. SUITABLE FOUNDING SOILS WOULD NORMALLY BE RESIDUAL SANDSTONE OR OTHER DENSE CLAYEY SANDS. CLEAN SANDS, SILTY SANDS OR HIGH CLAY CONTENT SOILS SUCH AS RESIDUAL SHALE ARE GENERALLY UNACCEPTABLE AS FOUNDING MATERIALS FOR ROCK RETAINING WALLS. NOTWITHSTANDING THE ABOVE A FOUNDATION SAFE BEARING CAPACITY OF 200KPA MUST BE ACHIEVED PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.
- 7. DRAINAGE OF THE WALL FOUNDATION SHALL BE ACHIEVED BY INSTALLING A CONTINUOUS A 100MM
- 8. DIAMETER SUBSOIL DRAIN AT THE REAR OF THE WALL WHERE THE WALL FOUNDATION CONSISTS OF SOIL.

 ALL SURFACE RUNOFF SHALL BE DIRECTED AWAY FROM THE BACK OF THE WALL SO AS TO PREVENT INFILTRATION OF SUCH SURFACE RUNOFF INTO THE GRANULAR BACKFILL.
 IN THE CASE OF WALLS FOUNDED ON SOIL THE SURFACE RUNOFF SHALL BE DIRECTED SO AS TO PREVENT EROSION AND

IN THE CASE OF WALLS FOUNDED ON SOIL THE SURFACE RUNOFF SHALL BE DIRECTED SO AS TO PREVENT EROSION AND POSSIBLE UNDERCUTTING ALONG THE TOE OF THE WALL.

- 10. MATERIALS USED FOR CONSTRUCTION OF THE WALL SHALL BE LARGE, DURABLE BOULDERS, IN GENERAL AT LEAST 0.5 SQUARE METERS IN AREA
- 11. WHERE THE WALL IS FOUNDED ON BEDROCK THE PREPARATION OF THE FOUNDATION SHALL INCLUDE THE REMOVAL OF ALL LOOSE ROCK AND SOIL. ANY IRREGULARITIES IN THE LEVEL OF THE BEDROCK SHALL BE FILLED WITH MASS CONCRETE OR RIPPED ROCK COMPACTED TO A MINIMUM DRY DENSITY OF 98% STANDARD.
- 12. WHERE THE WALL MAY BE FOUNDED ON SOIL THE EXPOSED FOUNDATION SHALL BE SCARIFIED TO A MINIMUM DEPTH OF 300MM, WETTED TO BRING IT NEAR TO THE STANDARD OPTIMUM MOISTURE CONTENT AND COMPACTED TO A MINIMUM DRY DENSITY RATION OF 95% STANDARD. ANY AREA WHICH REMAINS SOFT OR SPONGY AFTER THE COMPACTION, SHALL BE EXCAVATED AND FILLED.
- 13. ALL FILL SHOULD BE PLACED IN LAYERS WITH A MAXIMUM LOOSE THICKNESS OF 250MM AND COMPACTED IN THE MANNER DESCRIBED ABOVE. THE BASE LAYER OF BOULDERS SHALL BE SET IN CONCRETE OR MORTAR PLACED ON THE PREPARED SOIL FOUNDATION WITHIN THE KEY EXCAVATION. WHERE THE ROCK SURFACE FALLS AWAY BELOW THE TOE OF THE WALL, PARTICULAR CARE SHALL BE TAKEN TO ENSURE THAT THE WALL IS FOUNDED ON INTACT BEDROCK AND NOT ON A FOUNDATION OF FLOATERS.
- 14. WHERE THE WALL IS TO BE FOUNDED ON BEDROCK THE BASE OF THE WALL SHALL BE NOTCHED IN SOUND, INSITU ROCK A MINIMUM DEPTH OF 100MM BELOW THE FINISHED SURFACE LEVEL AT THE TOE OF THE WALL. WHERE ADDITIONAL RESISTANCE TO SLIDING IS REQUIRED THE BASE OF THE WALL SHALL BE LAID INTO THE FOUNDATION ROCK BY CONCRETING THE BASECOURSE OF BOULDERS.
- 15. WHERE THE WALL IS TO BE FOUNDED ON A COMPACTED SOIL THE BASE OF THE WALL SHALL BE AT A LEVEL WHICH IS A MINIMUM DEPTH OF 450MM BELOW THE FINISHED SURFACE LEVEL AT THE TOE OF THE WALL.
- 16. ROCK IS TO BE PLACED TO ENSURE THAT INDIVIDUAL BLOCKS ARE INTERLOCKING. TO ACHIEVE THIS, BLOCKS SHOULD BE LAID ROUGILLY COURSED AND BEDDED ON THEIR DROADEST DASES. ALL VERTICAL JOINTS DETWEEN DLOCKS SHALL DE DROKEN FILTER CLOTH SHALL BE PLACED BEHIND THE ROCK WALL AND UNDER THE BASE.
- 17. MATERIALS USED FOR BACKFILLING BEHIND THE WALL SHALL BE ESSENTIALLY GRANULAR FREE MATERIAL
- 18. ANY COMPACTION OF FILL PLACED BEHIND THE WALL SHALL BE CAREFULLY CARRIED OUT TO MINIMISE THE INDUCED LATERAL STRESS AGAINST THE WALL.
- 19. ALL FILL SHOULD BE PLACED IN LAYERS WITH A MAXIMUM LOOSE THICKNESS OF 250MM AND COMPACTED TO A DRY DENSITY RATIO OF NOT LESS THAN 95% BASED ON STANDARD COMPACTION.
- 20. WHERE PAVEMENT CONSTRUCTION IS TO TAKE PLACE USING THE BACKFILL AS SUBGRADE MATERIAL, NEAR SURFACE COMPACTION OF THE BACKFILL SHALL BE CARRIED OUT IN ACCORDANCE WITH PAVEMENT COMPACTION REQUIREMENTS AND SURCHARGE LOADS BY COMPACTION MACHINERY.
- 21. WHERE THE POTENTIAL EXISTS FOR SIGNIFICANT SURFACE RUNOFF OR SUBSURFACE GROUNDWATER FLOW, SUFFICIENT DRAINAGE MEASURES SHALL BE INCORPORATED TO ENSURE THAT WATER PRESSURE DOES NOT DEVELOP IN THE MATERIAL RETAINED BY THE WALL.
- 22. WHERE REQUIRED THE OVERALL STABILITY OF THE SLOPE AND ROCK WALL SHALL BE EVALUATED. SUCH AN EVALUATION MUST BE CARRIED OUT BY A SUITABLY QUALIFIED CIVIL ENGINEER, PRACTICING GEOTECHNICAL ENGINEERING, PRIOR TO COMMENCING PLACEMENT OF THE WALL ROCK.
- 23. WHERE PIPES ARE TO PASS THROUGH OR BENEATH THE ROCK WALL THEY SHALL BE ENCASED IN CONCRETE TO ENSURE THAT THE BASE OF THE WALL IS FOUNDED IN COMPETENT MATERIAL.
- 24. THE NON STANDARD CONDITIONS THAT WARRANT SPECIAL DESIGN CONSIDERATION ARE:
- (i) BACKFILL SLOPE STEEPER THAN 1(V) IN 4(H)
- (ii) UNIFORM SURCHARGE GREATER THAN 20KPA OR LARGE ISOLATED LOADS WITHIN A DISTANCE OF ONE WALL HEIGHT OF THE EDGE OF THE WALL
- (iii) BUILDINGS WITHIN A DISTANCE OF ONE WALL HEIGHT OF THE EDGE OF THE WALL
- (iv) FOUNDATION CONDITIONS OUTSIDE THOSE SET OUT IN NOTE 4
- (v) WALLS HIGHER THAN 7M
- (vi) WALLS IN DESIGNATED LANDSLIP AREAS.
- (vii) WALLS WHERE THE SITE CONDITIONS ARE SUCH THAT THE BACKFILL WILL NOT BE FULLY DRAINED

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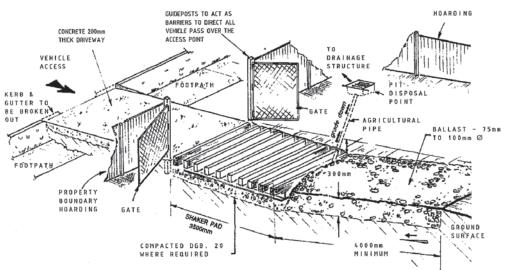
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STABILISED ACCESS POINT

TYPE II SAP

THE TYPE II SAP DESIGN IS MORE DEFINED IN THAT IT REQUIRES AN AREA OF BALLAST WITHIN THE SITE COMBINED WITH A SHAKER PAD; ADJACENT THE SHAKER PAD AND IN THE PUBLIC WAY IS A TEMPORARY (CONCRETE) VEHICULAR CROSSING. (SEE DIAGRAM)

STABILISED ACCESS POINT - TYPE 2



NOTES

IN BOTH TYPE I AND TYPE II SAPS, THE TEMPORARY VEHICULAR CROSSING MUST:

CONNECT TO AN EXISTING GUTTER LAYBACK (WHERE KERB AND GUTTER EXIST). IF A GUTTER LAYBACK DOES NOT EXIST THEN THE CONNECTION MUST BE MADE TO THE GUTTER BY REMOVING THE ADJACENT KERB SECTION ONLY.

CONNECT TO A DISH CROSSING (WHERE KERB AND GUTTER DOES NOT EXIST). IF A DISH CROSSING DOES NOT EXIST, THEN IT MUST BE CONSTRUCTED IN ACCORDANCE WITH DETAILS CONTAINED IN COUNCIL'S ISSUED FOOTPATH CROSSING LEVELS.

IT SHOULD BE NOTED THAT THESE TYPES OF SAPS ARE CONSIDERED TO BE APPLICABLE FOR THE MAJORITY OF ACTIVITIES HOWEVER SOME SITES MAY REQUIRE SPECIAL CONSIDERATION.

SHAKER PAD (CATTLE-GRID)

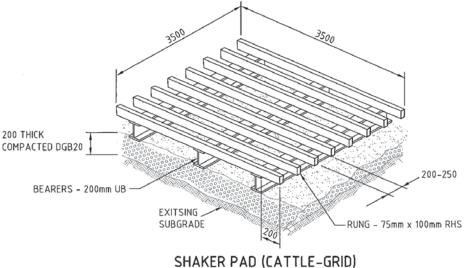
A CORRECTLY DESIGNED AND INSTALLED SHAKER PAD WILL ASSIST IN PREVENTING SEDIMENT TRANSFER FROM A SITE, ANY STABILISED ACCESS POINT (SAP) CAN BE DESIGNED WITH A SHAKER PAD (COMPULSORY IN TYPE II SAP'S).

SHAKER PADS CAN BE DESIGNED AND CONSTRUCTED TO ENABLE RE-USE ON FUTURE PROJECTS.

THE SHAKER PAD:

- MUST BE DESIGNED AND CERTIFIED BY A PRACTICING STRUCTURAL ENGINEER. THE CERTIFIED DESIGN SHOULD BE SUBMITTED WITH THE RELEVANT APPLICATION
- CAN BE CONSTRUCTED FROM ANY SUITABLE MATERIAL
- MUST BE LOCATED ON A SUITABLY PREPARED AND COMPACTED SUB-GRADE/BASE MATERIAL
- MUST BE SITUATED SUCH THAT THE RUNGS OF THE SHAKER PAD ARE LEVEL WITH THE ADJOINING NATURAL SURFACE
- MUST BE A MINIMUM 3.5M IN LENGTH
- . MUST BE A MINIMUM 3.5M IN WIDTH
- MUST HAVE CLEAR SPACING BETWEEN RUNGS OF 200 250MM
- RUNGS MUST HAVE A MAXIMUM WIDTH (BEARING AREA) OF 75MM
- MUST HAVE A MINIMUM CLEAR DEPTH OF 300MM IE FROM THE TOP OF THE RUNG TO THE FINISHED SUB-GRADE/BASE LEVEL

THE SHAKER PAD MUST BE PROVIDED WITH SUITABLE BARRIERS AT THE SIDES TO ENSURE THAT ALL TYRES OF VEHICLES LEAVING THE SITE TRAVERSE THE DEVICE.



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